Establishing links between performancebased durability design and sustainability

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Presentation overview

Contents Durability design/specification (with an eye on sustainability):

- Prescriptive approach
- Performance-based approach
 Deemed-to-satisfy approach
 Full- probabilistic approach
 Comparison of results

Links between durability and sustainability

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Introduction

Prescriptive specifications do not adequately address sustainability issues

- Do not have the flexibility in addressing new or marginal materials
- Conservative approach may result in resource waste

 Danger of under-specification for severe environments leading to premature failure
 No means to check actual specification

requirements, in particular the as-built quality



Introduction

Consequences:

- The resulting concrete is 'indeterminate' (in the sense that we do not really know what we have got)
- It may actually be inadequate to the task, thus requiring repair and maintenance during the structure's service life resulting in:
 - > Unanticipated material consumption
 - Social disruptions



Introduction

- On the other hand, it may be overconservative
- The irony is that, while many current durability specifications are viewed as being over-conservative, we have the phenomenon of a massive stock of under-performing infrastructure in respect of longevity and durability
- This is simply 'un-sustainable'



Performance-based Durability Design and Specification

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Performance-based design and specifications limit the environmental consequences on the structure to defined acceptable levels (targets) during the structure's service life (e.g. a pre-defined level of deterioration or state of repair)





General framework for performance-based design: **1.** A Robust Quality Control Test (or Tests) Routine, easily-carried out, reliable measures of resistance to deterioration (e.g. to chloride ingress) 2. A Service Life Model This must relate performance to the quality control test (e.g. in terms of limiting material parameters) **3.** Able to account for Differences (i.e. 'Margins') between 'Material Potential' and 'As-Built' Values In order to differentiate between areas of responsibility (e.g. material supplier & constructor) Next section illustrates the above with reference to SA developments

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1) Quality control tests:

– the approach developed in SA is that of *durability index (DI) tests* which serve as practical tools for durability studies, and to characterize the resistance of concrete to ionic or molecular transport



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2) Service-life models

- One SLM for chloride resistance, using 28day chloride conductivity as an input to a Fickian-type model
- One SLM for carbonation resistance, using 28-day OPI as a parameter - empirical
- Initiation models Account for material type and environment
- Link between DI parameters & use in SLM integrated approach





Integrated Design Framework & Methodology



Design methodology has been applied to two conditions (ref FIB Model Code):
Deemed-to-satisfy approach
Full- probabilistic approach





fib Model Code Framework



I) Deemed-to-satisfy approach

- 1. Performance criteria are drawn from the relevant SLM
- 2. The emphasis is on as-built concrete quality
- 3. Characteristic values of design parameters are used
- 4. Margin allowed between material 'potential' and as-built quality



cont... I) Deemed-to-satisfy approach

1. Performance criteria are drawn from the relevant service-life model

 C_x = chloride concentration at depth x and time t C_s = chloride surface concentration erf = mathematical error function D_c = diffusion coefficient

 $C_{x} = C_{s} \left[1 - erf \left[\frac{x}{2 \sqrt{D}} \right] \right]$

CC values: from < 0.5 to 3.5 mS/cm



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Cont... I) Deemed-to-satisfy approach

Example of performance criteria – CC values

Table 1: Example: chloride-conductivity, 100 y life

Marine Structure		Max. chloride conductivity (mS/cm) for		
50-yr design life		various binder types		
Exposure class	Cover	100% CEM	30% fly ash	50% Corex slag
(based on EN 206)	(mm)	Ι		
XS3b: Tidal, splash	40	0.45	0.75	1.05
and wetted spray	60	0.95	1.35	1.95
zones, exposed to abrasion	80	1.30	1.80	2.60

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Material Potential vs As-Built Values

2.-4. As-built quality; Char. Values; Margins allowed







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II) Full-probabilistic Approach

The methodology for a full-probabilistic design follows limit-state format given in ISO 13823

 which groups the design variables into either load (S) or resistance variables (R)

 Each variable is represented as a stochastic quantity

$$g(\mathbf{x}, \mathbf{C}_{_{\mathrm{crit}}}, \mathbf{C}_{_{\mathrm{s}}}, \mathbf{D}_{_{\mathrm{c}}}, t, m) = \mathbf{R} - \mathbf{S} = 0$$

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II) Full-probabilistic Approach

The design objective is to ensure that the calculated P_f for a given set of parameters does not exceed a target P_f given in the standards for a defined limit state

 $\mathbf{P}_{\mathrm{f}} = \mathbf{P} \{ \mathbf{R} - S < 0 \} < \mathbf{P}_{\mathrm{target}}$

and for the initiation limit state (ILS):

$$P_{f} = P\{C_{crit} - C_{(x,t)} < 0\} < P_{target, ILS}$$

where

$$C_{(x,t)} = C_{S} \left(1 - erf\left(\frac{x}{2\sqrt{D_{i}t^{(1-m)}}}\right) \right)$$

Solution of the LSF is carried out using Monte Carlo Simulation techniques that give the probability of failure

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Design Example



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Design Example

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blast furnace slag (GGBS)

Design life = 100 years







Design Example: Probabilistic approach

Statistical quantification of parameters in the LSF is carried out by fitting distribution curves to data obtained from in-situ testing. e.g. for C_S from profiling tests on existing RC structures (3-75 yrs) in SA

$$g(x, C_{crit}, C_{s}, D_{c}, t, m) = R - S = 0$$



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Using the statistical quantities for each parameter, a MCS of the LSF is done to get an output represented by a cumulative density function (CDF)

Table 2: Input parameters for reliability analysis of RC Pier

Parameter	Units	Mean	COV#	Distribution
Х	mm	50	0.2	Normal
C _{crit}	%	0.48	0.31	Normal
Cs	%	4.13	0.21	Gamma
t	years	100	-	Deterministic
m	_	0.68	_	Deterministic
COV # Coofficient	ofvariation			

COV[#] = Coefficient of variation

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Design Example: Probabilistic approach

MCS carried out using a Matlab sub-routine



Process is repeated a number of times $(10\ 000\ \text{iterations} = \text{N})$

If for a set of random values sampled LHS <0 then failure is said to occur & program registers this event (n)

P_f is then computed as:

$$P_{f} = \frac{n}{N}$$

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Design Example: Probabilistic approach



The probabilistic approach provides a means to deal with the variability in durability design parameters

The approach gives a more realistic representative of the 'real life' situation where variability is inherent, as compared to the deemed to satisfy or deterministic models





Deemed-to-satisfy vs Full Probabilistic



Diffusion coefficient (mm²/year)

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Performance-based approach

It can be argued that a purely deterministic approach should give a CC equivalent to 50% $\rm P_{f}$

Table 3: Design specifications (XS3b; Cover = 50mm $\pm \Delta x_{dev}$; CEM III A)

Design method	Chloride conductivity value
Deterministic (50% P _f)	1.92 mS/cm
Deemed-to-satisfy (DTS)	1.05 mS/cm
Full-probabilistic (6.7% P _f)	0.9 mS/cm



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Performance-based approach vs Prescriptive Approach

For the example, the table below compares three approaches to durability design

Table 4: Design specifications (XS3b; Cover = 50mm $\pm \Delta x_{dev}$; CEM III A)

Design method	Quality control	water/ binder	
	test value e.g. СС (СЕМ IIIA)	CEM IIIA	CEM I (Equiv.)
Prescriptive approach (EN 1992-1-1:2004)		0.4	-
Deemed-to-satisfy	1.05 mS/cm	0.75#/0.55	0.30
Full-probabilistic	0.9 mS/cm	0.70#/0.55	0.25

 $^{\#}$ = due to limitations in the current state of knowledge, a maximum w/b=0.55 is recommended

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Summary of Performance-based approach:

Quantification of the problem,
Idea of design 'targets', and
A design framework for engineers





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Durability design and Sustainability:

Inadequate durability leads to need for repair and rehabilitation - unnecessary waste of resources
 Should allow efficient use of resources
 Should allow utilization of new materials

BUT: how can we use the concepts of durability design to assist us to develop 'sustainability design'

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So far structural reliability theory has provided a *quantitative link* between the practice of structural engineering and its economic and societal consequences – mainly struc. safety

 Table 5: Consequences classes
 for SLS (FIB Model Code)

Consequences class*	Reliability level	Probability of
		failure (P _{target})
CC1 (Low)	1.0	16%
CC2 (Normal)	1.5	6.7%
CC3 (High)	2.0	2.3%

*The reliability level is selected based on consequences of failure i.e. If there will be many lives lost (public building) then a higher reliability level is selected

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The target prob. of failure (P_{target}) values are derived from optimizing the direct economic benefits/costs incurred by the owner/manager of the infrastructure.

- If the calculated P_f departs from P_{target} then the design is deemed uneconomical
- The P_{target} values at this stage relate to design standards established by developed countries; these may not relate to economic conditions faced in developing countries, e.g.
 - quality control levels might have to differ
 - 'development' may be more important than resource minimisation

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At present, the P_{target} value does not assign a value to the loss suffered by the society (user) when durability/structural failure occurs E.g. loss due to:
 Inconvenience during repair
 Loss of assets in case of structural failure
 Loss of income due to repair
 Possible loss of life





Likewise, the environmental consequences are equally important but so far have not been included in the analysis of P_{target} (or an equivalent formulation)

- Environmental consequences of durability failure may include:
 - Consumption of additional energy and resources due to repair
 - Generation of wastes during repair
 - Loss of the original materials



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We need to go a step further and estimate (using actual values) the societal and environmental consequences i.e. the explicit impact of durability/structural failure related to a particular target reliability These might vary from country to country depending on the state of development



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Previous cost minimization function:

$$Z(p) = C(p) + \left[F \times P_f \right]$$

P = design criteria
 C(p) = Design and construction cost
 F = Direct material losses due to failure

Proposed cost minimization function:

$$Z(p) = C(p) + \left[\left(F + S + E \right) \times P_f \right]$$

S = Societal cost due to failure

E = Environmental cost

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Concluding comments

- To summarise: we have argued that considerations of sustainability should relate to concepts embedded in service life and performance-based design requirements of structures
- This would in turn allow for the comparison of different material design options on the basis of their performance, as opposed to the current practice of comparing their costs and/or environmental impacts without a common reference of performance
- Introduction of such a system will lead to the promotion of new environmentally friendly structures that meet economic and societal needs





Thank You



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